fb2

W

GENERAL NOTES:

- I. GIRDERS SHALL BE FABRICATED AND CONSTRUCTED IN ACCORDANCE WITH 2016 LOUISIANA STANDARD SPECIFICATIONS FOR ROADS AND BRIDGES.
- 2. SEE GIRDER DATA TABLE FOR CONCRETE CLASS, REQUIRED COMPRESSIVE STRENGTH AT 28 DAYS, STRENGTH AT RELEASE, AND OTHER DESIGN
- 3. PRESTRESSING STRANDS SHALL BE 0.6" NOMINAL DIAMETER UNCOATED, SEVEN-WIRE, LOW RELAXATION, GRADE 270 STRANDS CONFORMING TO SECTION 1009 OF THE STANDARD SPECIFICATIONS, UNLESS OTHERWISE NOTED.
- 4. THE STRANDS AT TOP OF GIRDER, (4) FOR LG-36 TO LG-72 AND (2) FOR LG-25, SHALL BE MIN. 3/8" NOMINAL DIAMETER, UNCOATED, SEVEN-WIRE, LOW RELAXATION, GRADE 270 STRANDS CONFORMING TO ASTM A416, STRESSED TO 10,000 LBS. EACH FOR LG-36 TO LG-72 AND 5,000 LBS. EACH FOR LG-25.
- 5. MINIMUM CONCRETE CLEAR COVER SHALL BE 2", UNLESS OTHERWISE NOTED.
- 6. REINFORCING STEEL BARS SHALL CONFORM TO SECTION 1009 OF THE STANDARD SPECIFICATIONS.
- 7. DIMENSIONS RELATED TO REINFORCING STEEL ARE OUT TO OUT OF BAR UNLESS OTHERWISE NOTED, DIMENSIONS RELATED TO REINFORCING STEEL SPACING ARE CENTER TO CENTER OF BARS.
- 8. WELDED WIRE REINFORCEMENT SHALL CONFORM TO SECTION 1009 OF THE STANDARD SPECIFICATIONS, UNLESS OTHERWISE SHOWN IN THE PLANS. THE DESIGN YIELD STRENGTH OF WWR IS 75 KSI.
- 9. THE CONTRACTOR IS RESPONSIBLE FOR STABILITY OF PRECAST PRESTRESSED CONCRETE GIRDERS DURING FABRICATION, STORAGE, TRANSPORTATION, ERECTION, AND DECK PLACEMENT. SUPPORTING ANALYSIS AND CALCULATIONS STAMPED, SIGNED, AND DATED BY A LOUISIANA LICENSED PROFESSIONAL ENGINEER AND SHOP DRAWINGS SHOWING THE METHOD OF LIFTING THE GIRDER, LIFTING LOCATIONS AND DETAILS, SUPPORT (DUNNAGE) LOCATIONS FOR STORAGE AND TRANSPORTATION DETAILS, AND ERECTION BRACING DETAILS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD (EOR) FOR REVIEW AND ACCEPTANCE PRIOR TO FABRICATION OF GIRDERS. GIRDERS ARE DESIGNED ASSUMING SUPPORTED ON DUNNAGE WITHIN 3.0 FEET FROM GIRDER ENDS.
 - ANY INHERENT STABILITY PROVIDED BY CAST-IN-PLACE DIAPHRAGMS SHALL NOT BE CONSIDERED BY THE CONTRACTOR IN DESIGNING THE REQUIRED CONSTRUCTION BRACING. THE DIAPHRAGMS ARE PROVIDED TO RESTRAIN LATERAL MOVEMENT OF GIRDERS WHEN THE BRIDGE IS IN-SERVICE AND ARE NOT INTENDED OR ALLOWED FOR USE AS CONSTRUCTION STABILITY BRACING.
- IO. SHOW THE DATE OF CASTING ON THE WEB OF EACH GIRDER NEAR THE DOWNSTATION END (END A). FOR EXTERIOR GIRDERS, SHOW THE DATE OF CASTING ON THE INTERIOR SIDE OF THE WEB. THE DATE OF CASTING SHALL ALSO BE RECORDED IN CAMBER DATA TABLE. SEE NOTE II FOR MORE
- II. THE CAMBER DESIGN DATA (CI, C2, C3, AND D5) SHOWN IN CAMBER DATA TABLE ARE ESTIMATED VALUES. IT IS CONTRACTOR'S RESPONSIBILITY TO MONITOR CAMBER GROWTH AND RECORD THE FIELD MEASURED DATA (MCI, MCIa, MC2, fb/, fb/a, fb/a, Eb/a, AND Eb2) IN CAMBER DATA TABLE. THE CAMBER DATA TABLE INCLUDING CAMBER DESIGN DATA, FIELD MEASURED DATA, AND THE DATES OF GIRDER CASTING AND RISER POUR SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND ACCEPTANCE AT LEAST FOURTEEN DAYS PRIOR TO RISER POUR. WHEN THE MEASURED "MC1" OR "MC2" DIFFER MORE THAN 1/2" (+ OR -) FROM "C1" OR C2", THE ENGINEER OF RECORD SHALL BE NOTIFIED IMMEDIATELY TO INVESTIGATE PROPER CORRECTIVE MEASURES. THE CONTRACTOR IS RESPONSIBLE FOR PERFORMING CORRECTIVE MEASURES AT NO ADDITIONAL COST TO THE LADOTD.
- 12. THE CONTRACTOR IS RESPONSIBLE FOR PREPARING 4" OR 6" CONCRETE TEST CYLINDERS AS REQUIRED TO DETERMINE THE MODULES OF ELASTICITY (Ebi, Ebia, AND Eb2) AND BREAK STRENGTH (fbi, fbia, AND fb2) IN ACCORDANCE WITH ASTM C469 AND ASTM C39, RESPECTIVELY. THE COST OF THIS WORK SHALL BE INCLUDED IN PAYMENT FOR GIRDERS.
- 13. PAYMENT FOR GIRDERS SHALL BE IN ACCORDANCE WITH SECTION 805 OF STANDARD SPECIFICATIONS.
- 14. PAYMENT FOR BEARING PADS SHALL BE IN ACCORDANCE WITH SECTION 814 OF STANDARD SPECIFICATION.

DEFINITIONS:

CI	ESTIMATED INITIAL	GIRDER	CAMBER	DUE T	O PRESTRESS	FORCE A	ND GIRDER	SELF-WEIGHT	AT TRANSFER

ESTIMATED GIRDER CAMBER AT ERECTION C2

C3 ESTIMATED FINAL GIRDER CAMBER

ESTIMATED TOTAL DEAD LOAD DEFLECTION D5

D DIAMETER OF ROUND BEARING PAD

MAXIMUM JOINT OPENING, MEASURED IN THE DIRECTION OF TRAVEL AT MINIMUM TEMPERATURE \triangle_{MAX}

CONCRETE MODULUS OF ELASTICITY MEASURED AT 18 HOURS AFTER RELEASE Ebl

CONCRETE MODULUS OF ELASTICITY MEASURED AT 28 DAYS Ebla

Eb2 CONCRETE MODULUS OF ELASTICITY MEASURED AT 21 DAYS BEFORE RISER POUR

DOWNSTATION END OF GIRDER FND A

END B UPSTATION END OF GIRDER

COMPRESSIVE CONCRETE BREAK STRENGTH MEASURED AT 18 HOURS AFTER RELEASE fb1

COMPRESSIVE CONCRETE BREAK STRENGTH MEASURED AT 28 DAYS fbla

COMPRESSIVE CONCRETE BREAK STRENGTH MEASURED AT 21 DAYS BEFORE RISER POUR

TOTAL HEIGHT OF GIRDER Η

HW

DISTANCE FROM END OF GIRDER TO CENTERLINE OF BEARING PAD MEASURED PERPENDICULAR TO THE CENTERLINE OF BEARING PAD

DISTANCE FROM END OF GIRDER TO CENTERLINE OF END DIAPHRAGM MEASURED ALONG THE CENTERLINE OF GIRDER WEB

LENGTH OF BEARING PAD

GIRDER LENGTH LG

LENGTH OF EMBEDDED & BEVELED PLATES LP

LENGTH OF EMBEDDED & BEVELED PLATES AT CLIPPED SIDE LPC LT DISTANCE FROM END OF GIRDER TO STRAND TIE DOWN POINT GIRDER CAMBER MEASURED AT 18 HOURS AFTER RELEASE

MC I GIRDER CAMBER MEASURED AT 28 DAYS MCIa

GIRDER CAMBER MEASURED AT 21 DAYS BEFORE RISER POUR MC2

TOTAL NUMBER OF SPACINGS FOR BARS 501 IN EACH OF THE SHEAR ZONES I TO 4, RESPECTIVELY NI, N2, N3, N4

VERTICAL BEVEL DIMENSION AT ENDS OF GIRDER

SLOPE OF GIRDERS DETERMINED ALONG THE CENTERLINE OF THE GIRDER AT THE CENTERLINE OF BEARINGS SL

SKEW ANGLE BETWEEN CENTERLINE OF SUPPORT AND A LINE NORMAL TO ROADWAY CENTERLINE

THICKNESS OF BEVELED PLATE AT THICKER END TB TOTAL THICKNESS OF ELASTOMERIC BEARING PAD

VI, V2, V3, V4 SPACING OF BARS 501 IN EACH OF SHEAR ZONES I TO 4, RESPECTIVELY

DISTANCE FROM CENTERLINE OF JOINT TO END OF GIRDER ALONG CENTERLINE OF GIRDER. TYPICAL 3 INCHES FOR NON-SKEWED JOINTS

AND SKEWED JOINTS AT LINK SLAB WITH SAME-DEPTH GIRDER. VARIES FOR OTHER SKEWED CONDITIONS.

HAUNCH THICKNESS AT GIRDER CENTERLINE AND AT CENTER OF BEARING

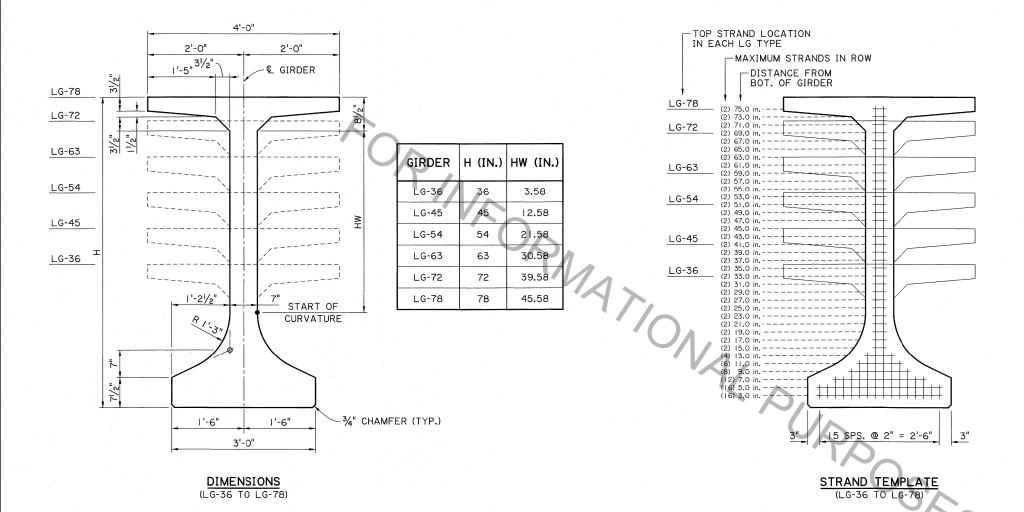
LG GIRDER SPECIAL DETAILS INDEX

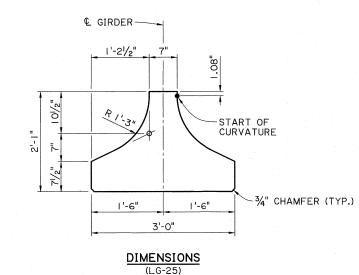
	BRIDGE STANDARDS INDEX NO.	SERIES	DESCRIPTION				
	BD.2.2.4.1.01	I OF II	INDEX, GENERAL NOTES AND DEFINITIONS				
	BD.2.2.4.1.02 2 OF 11		DIMENSIONS AND STRAND TEMPLATES				
	BD.2.2.4.1.03	3 OF 11	END OF GIRDERS				
L	BD.2.2.4.1.04	4 OF 11	END OF GIRDERS				
4	BD.2.2.4.1.05	5 OF 11	STANDARD STEEL-REINFORCED BEARING PADS				
	BD.2.2.4.1.06	6 OF 11	NON-STANDARD STEEL-REINFORCED BEARING PADS				
2	BD.2.2.4.1.07	7 OF 11	EMBEDDED AND BEVELED PLATES - SQUARE END OF GIRDER				
5	BD.2.2.4.1.08	8 OF 11	EMBEDDED AND BEVELED PLATES - CLIPPED END OF GIRDER				
		9 OF 11	COIL INSERTS AND PREFORMED HOLES FOR DIAPHRAGMS				
	BD.2.2.4.1.10	10 OF 11	CAMBER DETAILS				
	BD.2.2.4.1.11	II OF II	MISC. LG DETAILS				
LG-25	BD.2.2.4.2.01	I OF 2	LG-25 REINFORCEMENT DETAILS - CONVENTIONAL				
	BD.2.2.4.2.02	2 OF 2	LG-25 REINFORCEMENT DETAILS - WWR				
36	BD.2.2.4.3.01	1 OF 2	LG-36 REINFORCEMENT DETAILS - CONVENTIONAL				
- 5	BD.2.2.4.3.02	2 OF 2	LG-36 REINFORCEMENT DETAILS - WWR				
45	BD.2.2.4.4.01	I OF 2	LG-45 REINFORCEMENT DETAILS - CONVENTIONAL				
LG.	BD.2.2.4.4.02	2 OF 2	LG-45 REINFORCEMENT DETAILS - WWR				
54	BD.2.2.4.5.01	I 0F 2	LG-54 REINFORCEMENT DETAILS - CONVENTIONAL				
ار 9	BD.2.2.4.5.02	2 OF 2	LG-54 REINFORCEMENT DETAILS - WWR				
-63	BD.2.2.4.6.01	I OF 2	LG-63 REINFORCEMENT DETAILS - CONVENTIONAL				
Ġ	BD.2.2.4.6.02	2 OF 2	LG-63 REINFORCEMENT DETAILS - WWR				
-72	BD.2.2.4.7.01	I OF 2	LG-72 REINFORCEMENT DETAILS - CONVENTIONAL				
L Ģ	BD.2.2.4.7.02	2 OF 2	LG-72 REINFORCEMENT DETAILS - WWR				
-78	BD.2.2.4.8.01	1 OF 2	LG-78 REINFORCEMENT DETAILS - CONVENTIONAL				
ادًا	BD.2.2.4.8.02	2 OF 2	LG-78 REINFORCEMENT DETAILS - WWR				
	LG-72 LG-63 LG-54 LG-45 LG-36 LG-25	STANDARDS INDEX NO. BD.2.2.4.1.01 BD.2.2.4.1.03 BD.2.2.4.1.04 BD.2.2.4.1.05 BD.2.2.4.1.06 BD.2.2.4.1.08 BD.2.2.4.1.09 BD.2.2.4.1.10 BD.2.2.4.1.10 BD.2.2.4.1.10 BD.2.2.4.1.10 BD.2.2.4.1.10 BD.2.2.4.1.01 BD.2.2.4.3.01 BD.2.2.4.3.02 BD.2.2.4.4.01 BD.2.2.4.5.01 BD.2.2.4.5.01 BD.2.2.4.5.02 BD.2.2.4.6.01 BD.2.2.4.6.02 BD.2.2.4.7.01 BD.2.2.4.7.02 BD.2.2.4.7.02 BD.2.2.4.7.02	STANDARDS INDEX NO. BD.2.2.4.1.01 0F 11 BD.2.2.4.1.02 2 0F 11 BD.2.2.4.1.03 3 0F 11 BD.2.2.4.1.04 4 0F 11 BD.2.2.4.1.05 5 0F 11 BD.2.2.4.1.06 6 0F 11 BD.2.2.4.1.07 7 0F 11 BD.2.2.4.1.09 9 0F 11 BD.2.2.4.1.10 10 0F 11 BD.2.2.4.1.11 11 0F 11 BD.2.2.4.1.11 11 0F 11 BD.2.2.4.2.01 1 0F 2 BD.2.2.4.3.01 1 0F 2 BD.2.2.4.3.01 1 0F 2 BD.2.2.4.4.01 1 0F 2 BD.2.2.4.5.01 1 0F 2 BD.2.2.4.6.01 1 0F 2 BD.2.2.4.6.01 1 0F 2 BD.2.2.4.7.01 1 0F 2				

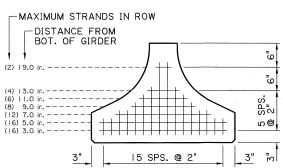




GENERAL NOT







STRAND TEMPLATE (LG-25)

NOTES:

I. SEE LG COMMON DETAILS SHT. I OF II FOR GENERAL NOTES AND DEFINITIONS.

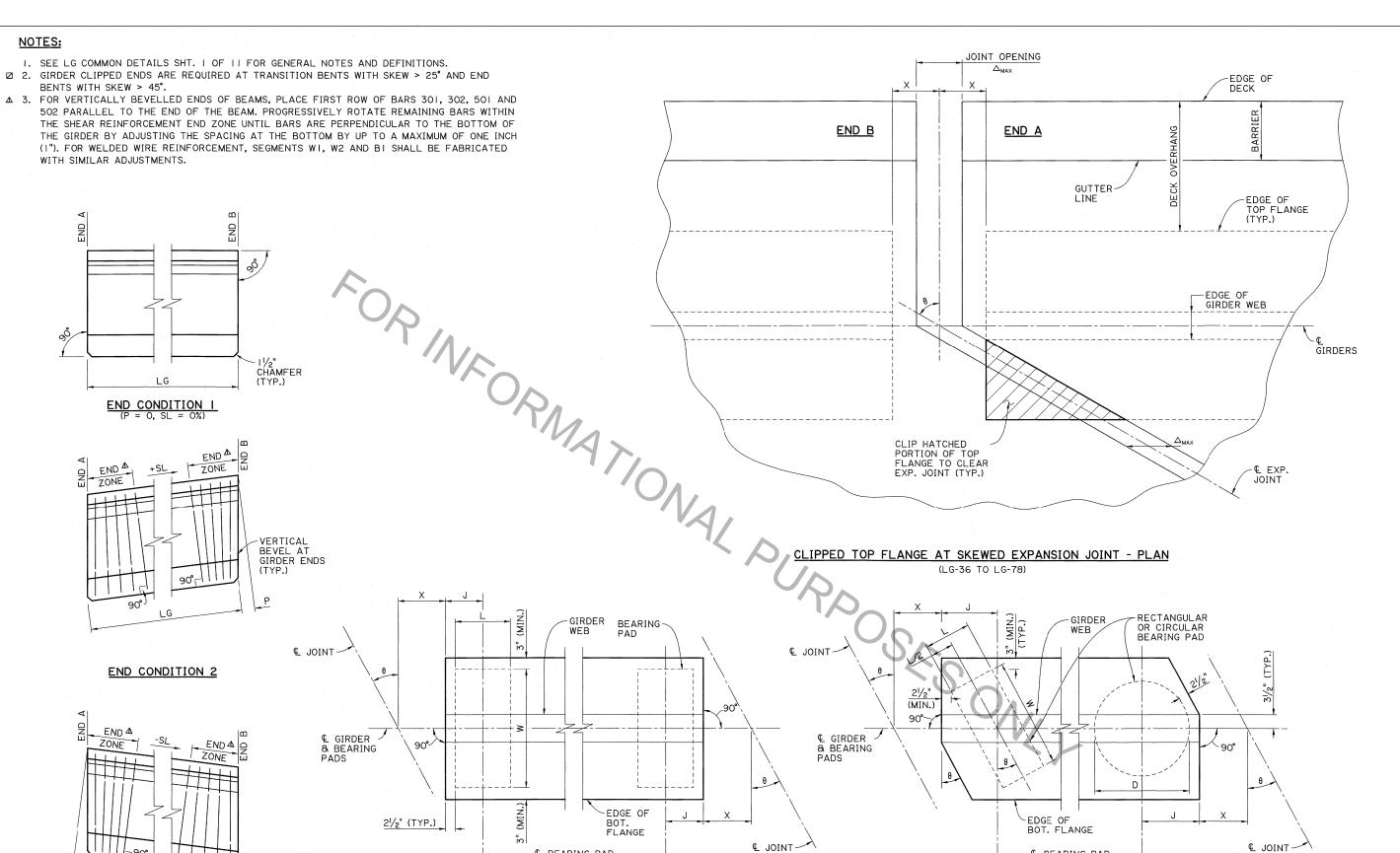




DIMENSIONS AND STRAND TEMPLATES

CONTROL SECTION STATE PROJECT

2. LIANG
B. KEVER
J. W. P.
Y. OUYANG
Z.Z. FU



END CONDITION 3

90°

PLAN AT GIRDER ENDS
(TOP FLANGE NOT SHOWN FOR CLARITY)

− L BEARING PAD-

GIRDER CLIPPED END

END B

END A



END B

- € BEARING PAD-

GIRDER SQUARE END

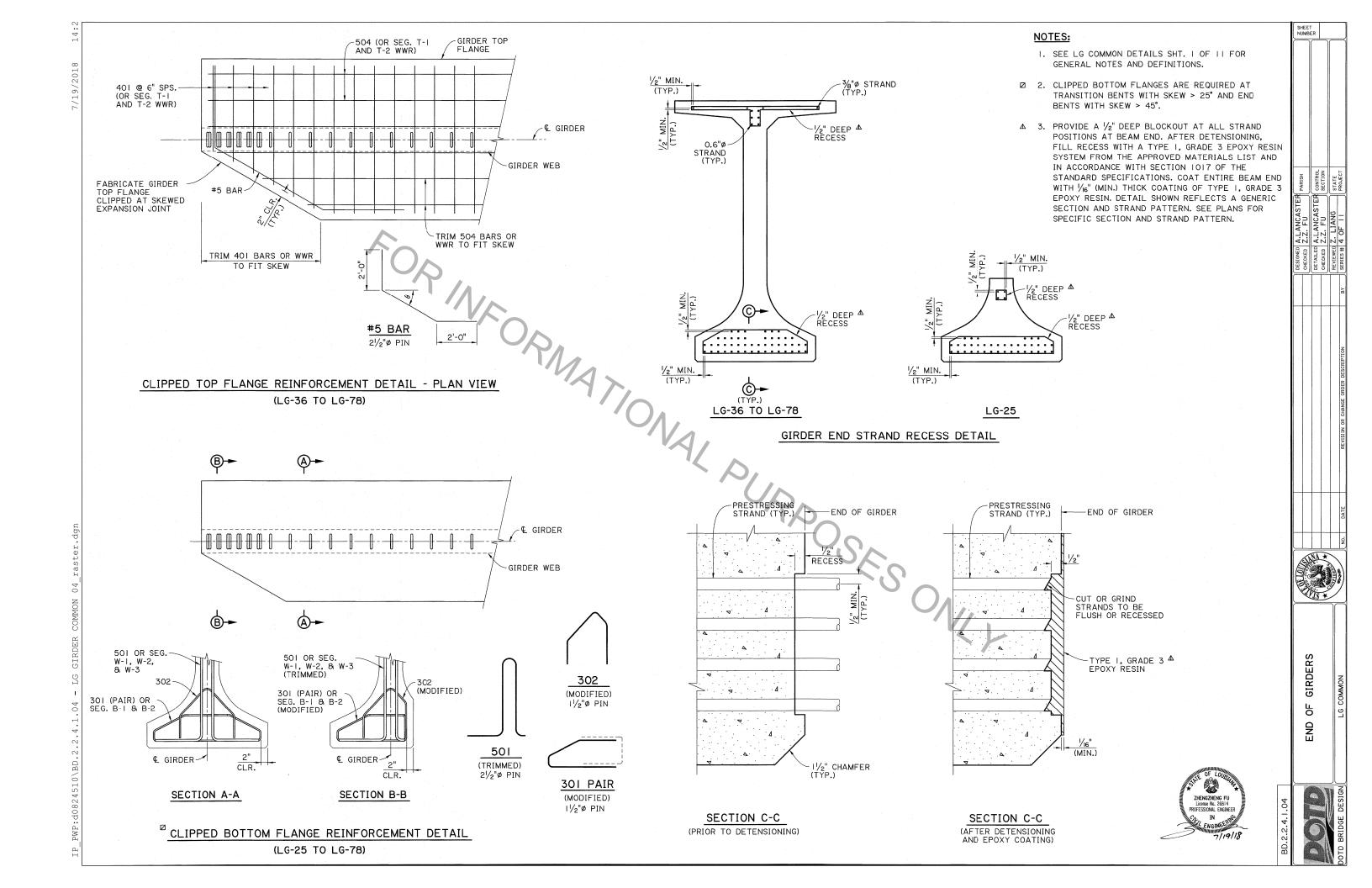
END A



GIRDERS

R

(LG-36 TO LG-78 SHOWN, LG-25 SIMILAR)



NOTES:

- SEE LG COMMON DETAILS SHT. I OF II FOR GENERAL NOTES AND DEFINITIONS.
- 2. ELASTOMER FOR ALL BEARING PADS SHALL HAVE A SPECIFIED SHEAR MODULUS (G) OF 150 PSI.
- 3. STEEL PLATES OF ELASTOMERIC BEARING PADS SHALL BE $1/\!\!/_{\theta}$ STEEL PLATES CONFORMING TO ASTM A36.

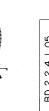
€ BEARING PAD € GIRDER <u>PLAN</u>

V ₈ " STEEL P	LATES	1/2" INT. ELASTO LAYER (TYP
118		<u> </u>
	1/4" ELASTOMER COVER (TYP.) $W = 2^1-6$ "	ELASTOMER R (TYP.)
	SECTION	4" EXT. ELASTO LAYER (TYP.)

STANDARD	STEEL-REI	NFORCED	ELASTOMERIC	BEARING PAD
	(TYPES B-I	TO B-9)	

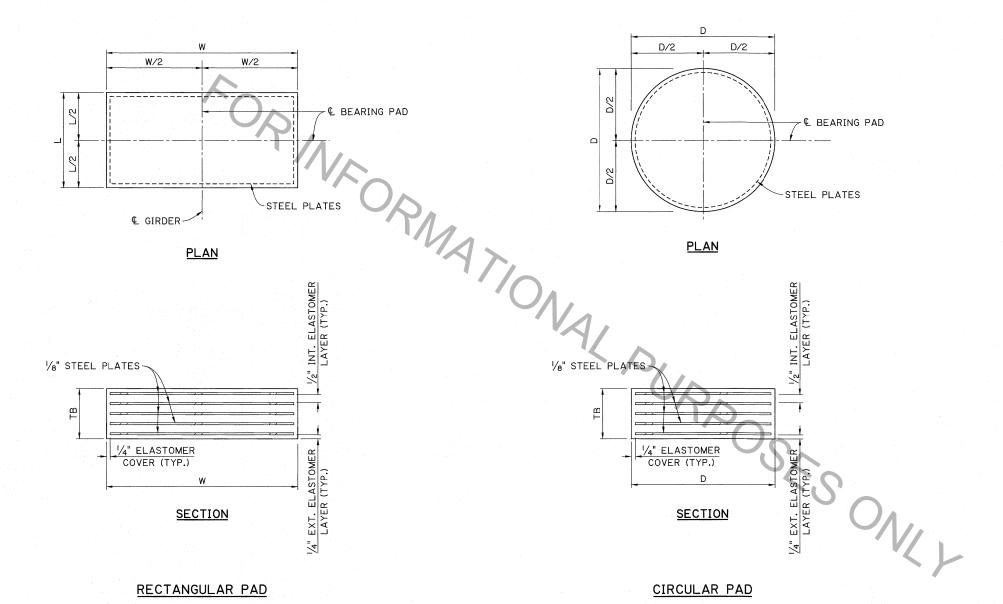
STEEL PLATES	STANDARD STEEL-REINFORCED ELASTOMERIC BEARINGS DATA TABLE						
NT. ELASTOMER AYER (TYP.)	TYPE	L	W	ТВ	NO. OF INTERIOR ELASTOMER LAYERS	NO. OF 1/8" STEEL PLATES	
	B-I	8"	2'-6"	17/8"	2	3	
	B-2	10"	2'-6"	17/8"	2	3	
	B-3	10"	2'-6"	21/2"	3	4	
INT. ELASTOMER LAYER (TYP.)	B-4	10"	2'-6"	31/8"	4	5	
TSI T	B-5	1'-0"	2'-6"	33/4"	5	6	
	B-6	1'-0"	2'-6"	43/8"	6	7	
4. 	B-7	1'-0"	2'-6"	5"	7	8	
NT. NT. LAYE	B-8	1'-2"	2'-6"	5%"	8	9	
=2	B-9	1'-2"	2'-6"	61/4"	9	10	
/4" EXT. ELASTOMER LAYER (TYP.)			OSE	S	0/1/2		
TOMERIC BEARING PAD -9)							





STANDARD STEEL-REINFORCED ELASTOMERIC BEARINGS

- SEE LG COMMON DETAILS SHT. I OF II FOR GENERAL NOTES AND DEFINITIONS.
- 2. ELASTOMER FOR ALL BEARING PADS SHALL HAVE A SPECIFIED SHEAR MCDULUS (G) OF 150 PSI.
- 3. STEEL PLATES OF ELASTOMERIC BEARING PADS SHALL BE $1/\!\!/_8$ STEEL PLATES CONFORMING TO ASTM A36.
- 4. SEE PROJECT CONTRACT PLANS FOR NON-STANDARD BEARING TYPES AND DATA TABLE.

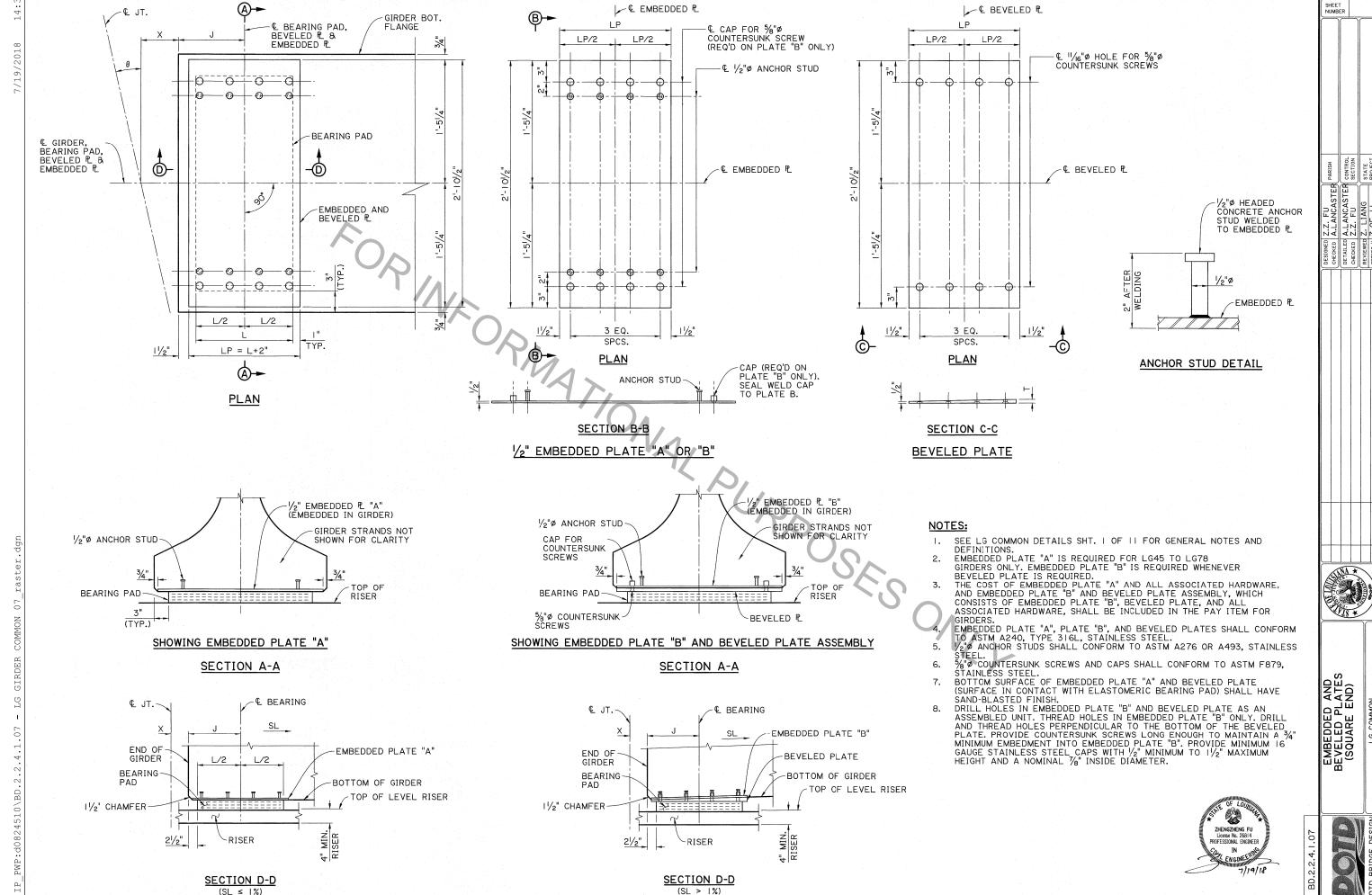


NON-STANDARD STEEL-REINFORCED ELASTOMERIC BEARING PADS





NON-STANDARD STEEL-REINFORCED ELASTOMERIC BEARINGS



EMBEDDED AND BEVELED PLATES (CLIPPED END)

EXTERIOR

SIDE

-€ I"Ø COIL INSERT FOR I"Ø x 4'-0"

THREADED ROD

DETAIL B

(@ L DIAPHRAGM)

EXTERIOR GIRDER

(LG-36 TO LG-78)

EXTERIOR GIRDER

(LG-25)

1"ø x 3'-0" ALL THREAD ROD

 \boxtimes 2"ø PREFORMED HOLE FOR #8 BAR (@ & DIAPHRAGM)

▼ THESE DIMENSIONS MAY BE
ADJUSTED TO CLEAR HARPED STRANDS WHEN NECESSARY. CONTACT ENGINEER WHEN CONFLICT OCCURS.

GIRDER TYPE

LG-36

LG-45

LG-54

LG-63

LG-72

LG-78

a⊠

12"

12"

12"

12"

11" 12" b⊠

N/A 11"

14"

23"

32"

38"

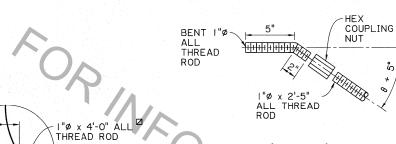
INTERIOR GIRDER

NOTES:

- £ 2"Ø PREFORMED HOLE FOR #8 BAR (@ £ DIAPHRAGM)

- I. SEE LG COMMON DETAILS SHT. I OF II FOR GENERAL NOTES AND
- COIL INSERTS SHALL BE WILLIAMS TYPE C17-1X4 THIN SLAB LIFTING INSERT OR AN APPROVED EQUAL.
- THE COST OF ALL HARDWARE SHALL BE INCLUDED IN THE COST OF GIRDERS.
- FABRICATOR SHALL CONTACT ENGINEER WHEN CONFLICT BETWEEN COIL INSERTS OR PREFORMED HOLES AND SHEAR REINFORCEMENT
- WHEN THE SKEW ANGLE EXCEEDS 30°, THE 2" PREFORMED HOLE IN THE WEB WILL CONFLICT WITH THE SHEAR REINFORCMENT ADJACENT TO THE HOLE. THE REBAR OR WWR MUST BE FABRICATED WIDER, AND ROTATED PARALLEL TO THE SKEW, IN ORDER TO ACCOMODATE THE 2" PREFORMED HOLE.
- FOR SKEWED DIAPHRAGMS, PROVIDE THREADED ROD IN TWO PIECES WITH A HEX COUPLING NUT (MINIMUM $2l_2^{\prime\prime}$ " IN LENGTH) TO AVOID CONFLICT BETWEEN THREADED ROD AND GIRDER FLANGE DURING INSTALLATION. SEE DETAIL "C".

 $\theta = 0^{\circ}$



LG-78

I"Ø x 4" COIL INSERT

DETAIL B

(LG-25)

0° < θ ≤ 60° DETAIL C

> INTERIOR GIRDER (LG-25)

(LG-36 TO LG-78)

8" END DIAPHRAGM 31/4" (MIN.) (TYP.) END OF "Ø x 4" COIL INSERT GIRDER £ EXTERIOR GIRDER GIRDER WEB LG-25 TO LG-72 $K = 1'-5\frac{1}{2}" (MIN.)^{\Delta}$ WEB SHEAR $K = 1'-11\frac{1}{2}" (MIN.)^{\Delta}$ REINFORCEMENT LG-78

- £ 8" END DIAPHRAGMS AND 2"Ø PREFORMED HOLE THROUGH GIRDER WEB WEB SHEAR -END OF REINFORCEMENT GIRDER - € INTERIOR GIRDER GIRDER WEB LG-25 TO LG-72 $K = 1'-5!/2'' (MIN.)^{\triangle}$ LG-25 TO LG-72 $K = 1'-11'/2'' (MIN.)^{\Delta}$ LG-78

€ 8" END DIAPHRAGMS AND 2"ø PREFORMED HOLE THROUGH GIRDER WEB WEB SHEAR O REINFORCEMENT GIRDER € INTERIOR GIRDER GIRDER WEB $K = 1'-51/2'' (MIN.)^{\triangle}$ $K = 1'-11'/2'' (MIN.)^{\Delta}$

END DIAPHRAGM LOCATION - PLAN VIEW

(EXTERIOR GIRDER) (N.T.S.)

END DIAPHRAGM LOCATION - PLAN VIEW

(INTERIOR GIRDER: $0^{\circ} \le \theta \le 30^{\circ}$) (N.T.S.)

END DIAPHRAGM LOCATION - PLAN VIEW

(INTERIOR GIRDER: $\theta > 30^{\circ}$) Θ (N.T.S.)

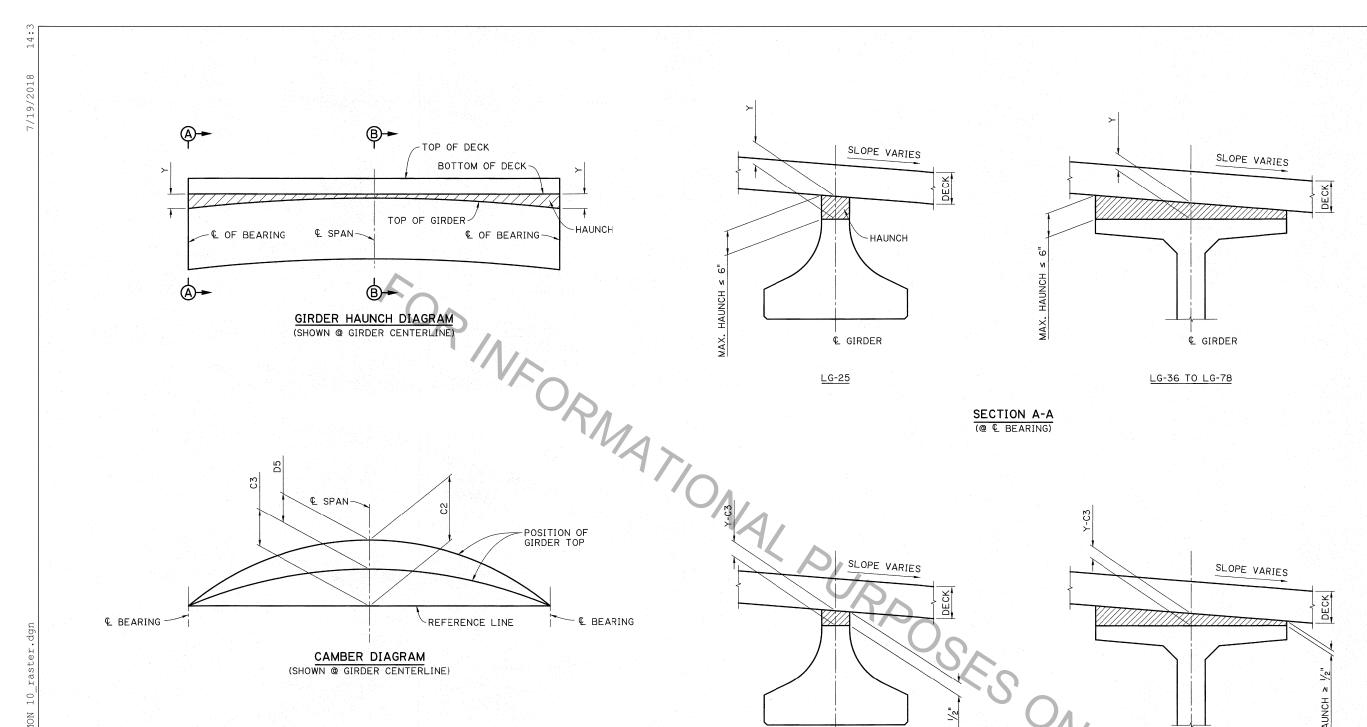


ALL DETAILS THIS SHEET : N.T.S.

COIL INSERTS
REFORMED HOLES
FOR DIAPHRAGMS

ďΣ

CONTROL SECTION STATE PROJECT



NOTES:

I. SEE LG COMMON DETAILS SHT. I OF II FOR GENERAL NOTES AND DEFINITIONS.

SECTION B-B
(@ © SPAN)

€ GIRDER

LG-36 TO LG-78

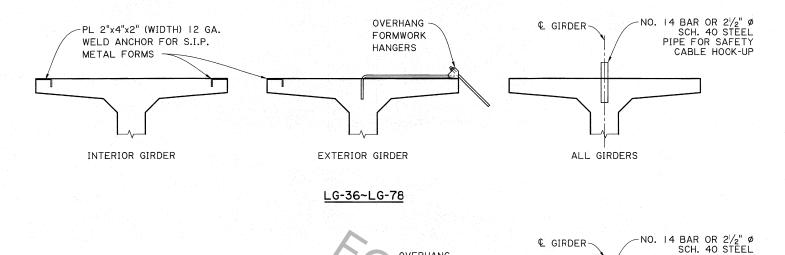
€ GIRDER

LG-25



BD.2.2.4.1.10

CAMBER DETAILS



-OVERHANG

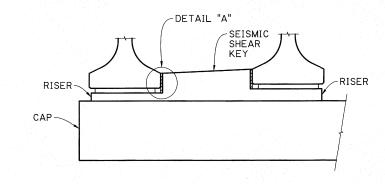
FORMWORK

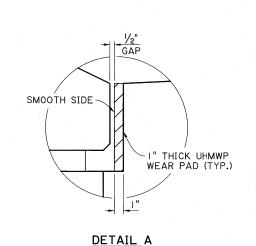
HANGERS

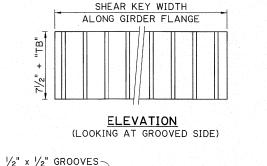
EXTERIOR GIRDER

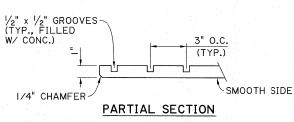
LG-25

FORMWORK & SAFETY CABLE ATTACHMENTS









△ ULTRA-HIGH MOLECULAR WEIGHT POLYETHYLENE (UHMWP) WEAR PAD AT SEISMIC SHEAR KEY

- I. SEE LG COMMON DETAILS SHT. | OF | | FOR GENERAL NOTES AND DEFINITIONS.
- ⊗ 2. SUGGESTED TRANSPORTATION TIE-DOWN METHOD SHOWN. SEE LG COMMON DETAILS SHEET | OF | | (NOTE 9) FOR CONTRACTOR RESPONSIBILITIES.

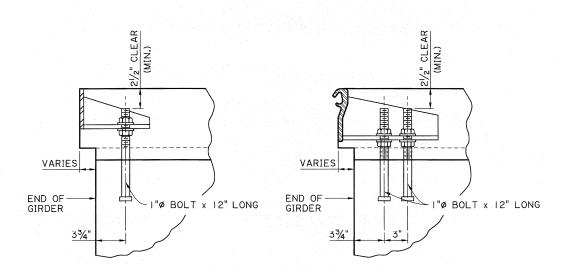
FIELD DRILLING OF HOLES IN WEB IS NOT PERMITTED. AFTER TRANSPORTATION, HOLES IN GIRDER WEB SHALL BE CLEANED OF FORM MATERIAL AND FORM RELEASE MATERIAL, THEN FILLED COMPLETELY WITH A NON-SHRINK GROUT THAT MATCHES OR EXCEEDS THE GIRDER CONCRETE STRENGTH.

- SEE MISC. SPAN DETAILS FOR JOINT INFORMATION NOT SHOWN HERE.
- PROVIDE WEAR PADS IN CONFORMANCE TO ASTM D6712-17 (CALLOUT S-UHMW-PEOIII). CHAMFER THE OUTER EDGES 1/4".

PLACE GIRDERS PRIOR TO CASTING SHEAR KEY. CAST WEAR PAD WITH SHEAR KEY, SMOOTH SIDE FACING GIRDER.

PAYMENT FOR WEAR PADS SHALL BE INCLUDED IN ITEM "CLASS AI CONCRETE (BENT CAP)".

(LG-36~LG-78)



PL 2"x4"x2" (WIDTH) 12 GA.

WELD ANCHOR FOR SIP METAL FORMS

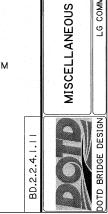
PREFORMED SILICONE

INTERIOR GIRDER

PREFORMED NEOPRENE

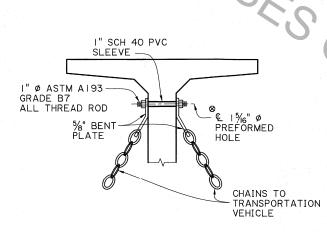
EXPANSION JOINT ANCHOR BOLT DETAIL (PARTIAL SECTION ALONG € OF GIRDER)





DETAILS

LG



PIPE FOR SAFETY

CABLE HOCK-UP

SUGGESTED TRANSPORTATION TIE-DOWN